GARNER ENGINEERING PA

Supplemental Soil Borings Stuttgart Industrial Park U.S. Hwy 79 Stuttgart, Arkansas

Report To

BETHANY HILDEBRAND
PRESIDENT & CEO
STUTTGART CHAMBER OF COMMERCE
507 SOUTH MAIN STREET
STUTTGART, AR 72160



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June 23, 2025

To: Bethany Hildebrand
President & CEO
Stuttgart Chamber of Commerce
507 South Main Street
Stuttgart, AR 72160

From: Carl W Garner P

Re: Supplemental Soil Borings

Stuttgart Industrial Park

Hwy 79

Stuttgart, Arkansas



As requested, the subsurface conditions have been further investigated by one (1) 30 ft deep SPT sample boring and one (1) 15 ft SPT sample boring. The results of these Supplemental Borings are summarized hereinafter.

- The borings were performed with hollow stem auger. Insitu samples were obtained via automatic hammer assisted Standard Penetration Tests (SPT). The location of 30 ft Boring A and 15 ft Boring B are shown on the Plan of Borings, Plate 1. The Plan of Borings also includes previous borings performed for the Geotechnical Study report dated December 16, 2024.
- Insitu soil strength and compressibility properties were evaluated by SPT N-values. N-values are plotted at appropriate depths on the logs in the Blows Per Foot column. Additionally, insitu cohesion estimates were obtained with a calibrated hand penetrometer. Cohesion estimates in Tons per Square Foot (TSF) are plotted on the logs using the symbol ⊗.
- All samples were field classified and subsequently examined in the laboratory for selective testing of engineering properties. Descriptions of soil strata encountered in the borings are presented in the Log of Boring, Plates 2 and 3. Results of selective laboratory tests i.e., Atterberg Limits and moisture content, are plotted on each Log of Boring.

Conclusions

The soils encountered to the completion depths of Borings A and B were found to be consistent with previous borings on the site and in the site vicinity. Accordingly, please refer

to our December 16, 2024 Geotechnical Report for detailed discussion of subsurface stratigraphy, engineering properties and recommendations for soil parameters for project design.

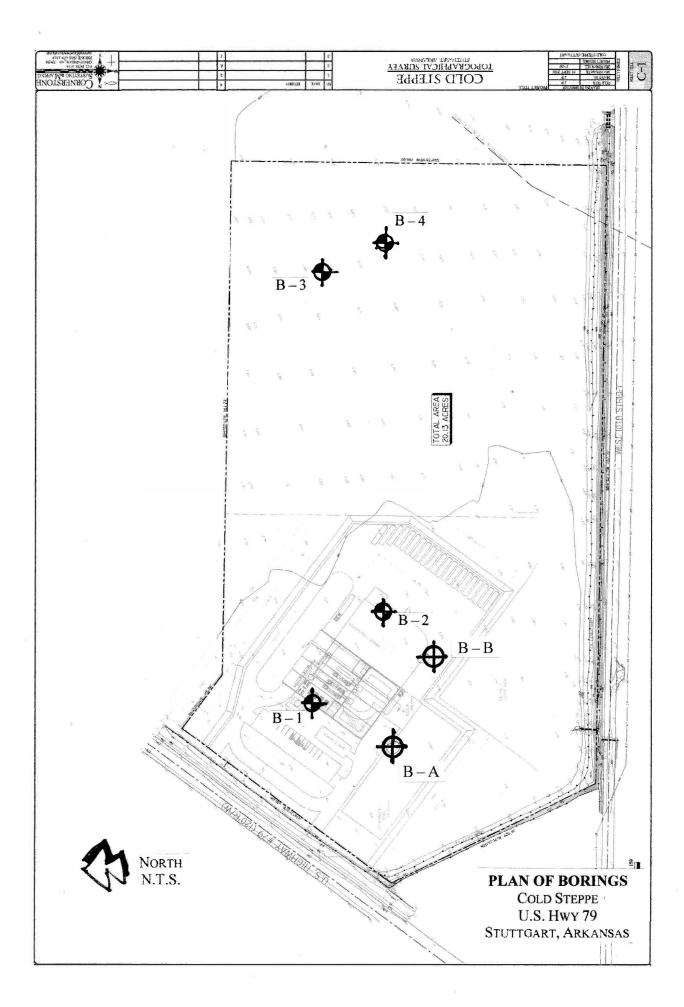
The following illustrations are attached and complete this report:

Plate 1 Plan of Borings Plates 2 and 3 Log of Boring

Plate 4 Key to Terms and Symbols

We trust the performance of Supplemental soil borings on the Cold Steppe site will assist in your project activity. However, if further information other than included in the soil and foundation study report should be required, please do not hesitate to call.

Cc: Bethany Hildebrand Matt Freniere Tommy Hoskyn



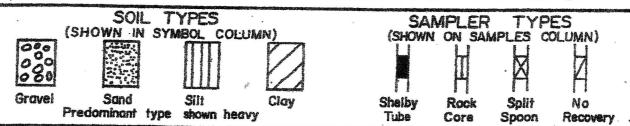
U.S. HWY 79 STUTTGART, ARKANSAS

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U.S. HWY 79 STUTTGART, ARKANSAS

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SYMBOLS AND TERMS USED ON BORING LOGS



TERMS DESCRIBING CONSISTENCY OR CONDITION

COARSE GRAINED SOILS(major portion retained on No. 200 sleve): includes (1) clean gravels and sands, and (2) sity or clayey gravels and sands. Condition is rated according to relative density, as determined by laboratory tests.

DESCRIPTIVE TERM	N-VALUE	RELATIVE DENSITY
VERY LOOSE LOOSE MEDIUM DENSE DENSE VERY DENSE	0-4 4-10 10-30 30-50 50 And o	0 - 15 % 15-35 % 35-65 % 65-85 % 1boye 85-100 %

FINE GRAINED SOILS (major portion passing No. 200 sieve): Includes (1) Inorganic and organic silts and clays, (2) gravelly, sandy, or silty clays, and (3) clayer silts. Consistency is rated according to shearing strength, as indicated by penetrometer readings or by unconfined compression tests.

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UNCONFINED COMPRESSIVE STRENGTH TON/SQ. FT.

VERY SOFT	Less than 0.25
SOFT	0.25 - 0.50
FIRM	0.50-1.00
STIFF	1.00 - 2.00
VERY STIFF	2.00-4.00
HARD	4.00 and higher

NOTE: Slickensided and fissured days may have lower unconfined compressive strengths than shown above, because of planes of weakness or cracks in the soil. The consistency ratings of such sails are based on penetrometer readings.

TERMS CHARACTERIZING SOIL STRUCTURE

SLICKENSIDED — having inclined planes of weakness that are slick and glossy in appearance.
FISSURED — containing strinkage cracks, frequently filled with fine sand or sitt; usually more or less vertical.

LAMINATED- composed of thin layers of varying color and texture.

INTERBEDDED - composed of alternate layers of different soil types.

CALCAREOUS - containing appreciable quantities of calcium carbonate.

WELL GRADED—having wide range in grain sizes and substantial amounts of all intermediate porticle sizes.

POORLY GRADED-predominantly of one grain size, or having a range of sizes with some intermediate sizes missing.

Terms used in this report for describing soils according to their texture or grain size distribution are in accordance with the UNIFIED SOIL CLASSIFICATION SYSTEM, as described in Technical Memorandum No.3-357, Waterways Experiment Station, March 1953.

Garner Engineering PA -----

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Phone: 501-225-8181 Mobile: 501-681-1491

501-681-1492

August 30, 2024

To: Matt Freniere Cold Steppe Inc

matt@coldsteppe.com

From: Carl W Garner

Re: Interim Report

Cold Steppe U.S. Hwy 79

Stuttgart, Arkansas

Bethany Hildebrand

Stuttgart Chamber of Commerce bethany@stuttgartchamber.com



This is the interim report of findings and preliminary recommendations developed from the geotechnical study on the site of the new Cold Steppe facility in Stuttgart, Arkansas. At this writing, specific building criteria and site grading plans are not yet available. Accordingly, this report summarizes basic geotechnical considerations for design development. The final report will be prepared when specific criteria and grading criteria become available.

- The geotechnical field investigation consisted of four (4) SPT sample borings. Two (2) borings were performed for the 110 ft x 180 ft facility and two (2) borings were performed in the 300 ft x 300 ft Tailwater Recovery Retention Pond and potential on site borrow area. The site layout was based on the "rough" site layout received July 31, 2024. Laboratory testing is currently ongoing to develop specific engineering properties.
- Based on subsurface conditions established from the borings and laboratory examination, the foundation soils may be generalized as follows.

Stratum I: Gray silt, ML, and clayey silt, ML-CL, was encountered to depths of

about 2 to 3.5 ft. These low plasticity soils were seasonally desiccated with apparent cohesion over 1.5 TSF. On the other hand, the soils are also moisture sensitive and subject to loss of strength and stability

during wet seasons.

Stratum II: Stratum I was underlain by a nominally 2 ft thick interval of very stiff

reddish brown trace gray clay, CL, CL-CH. This stratum is moderately plastic with cohesion typically over 1.2 TSF. Insitu water contents were

low reflecting moderate potential for swell related movement with cyclical or point source water content increase.

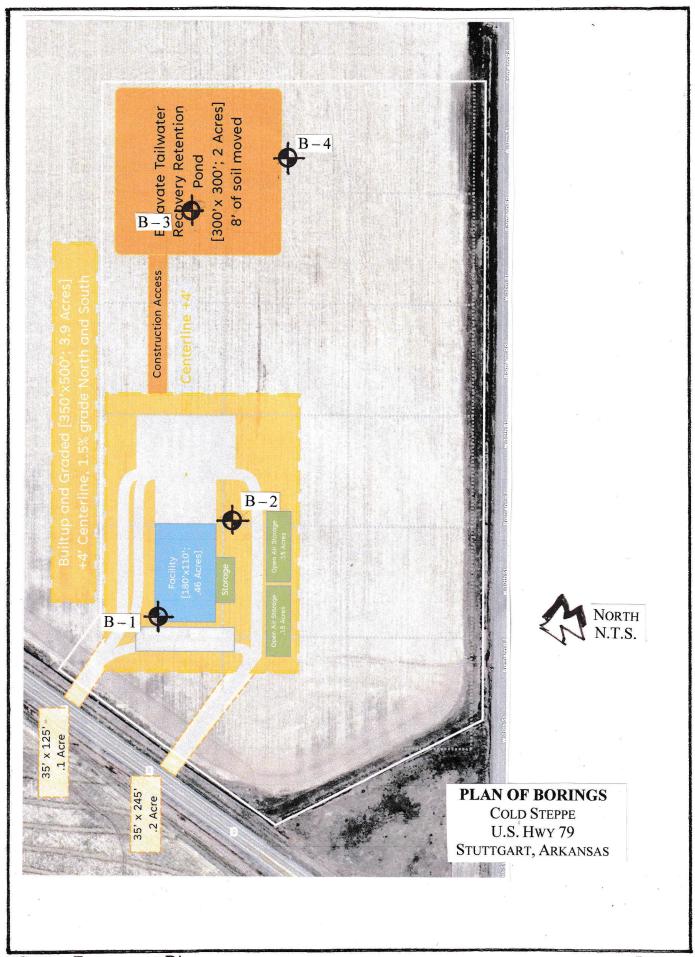
Stratum III: From depths of about 4 to 8 ft very stiff and stiff gray with brown clay, CL, CH, was encountered in all borings. This stratum exhibited variable but generally high moisture content with cohesion values of 0.6 to 1.5 TSF.

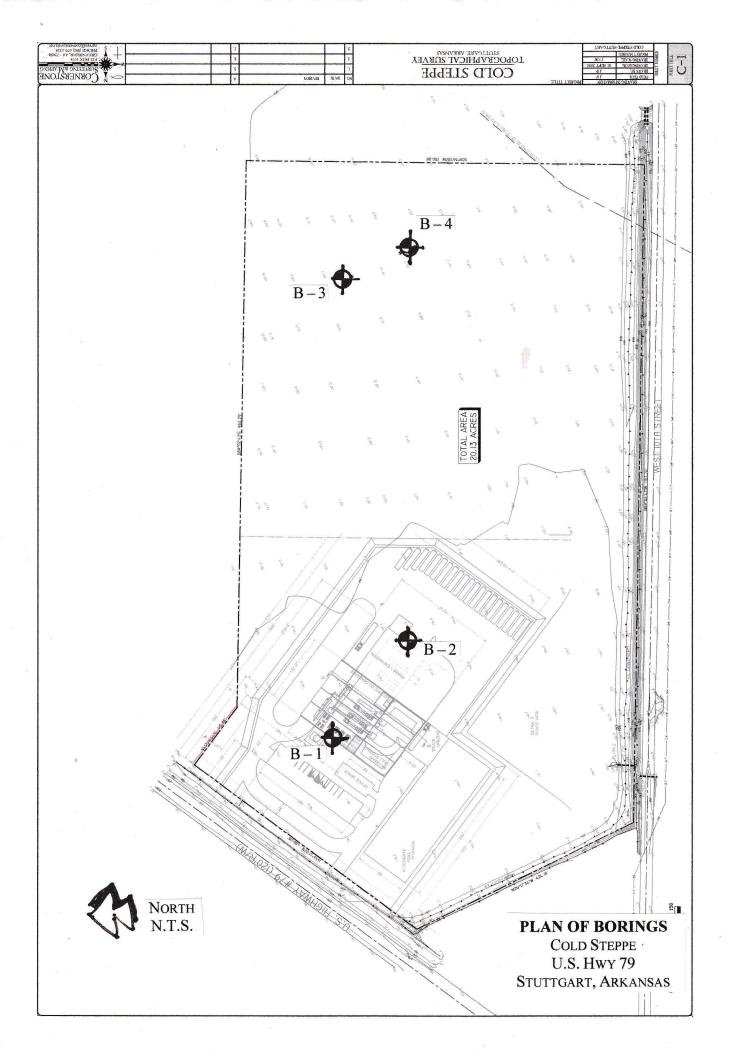
Stratum IV: The basal soil unit encountered below Stratum III consist of medium dense reddish tan silty fine sand, SM, grading to find sand, SP, with depth. N-values of 10 to 14 BPF were obtained in this dry sand interval.

Free groundwater was not encountered in the borings. However, the Stratum III clays exhibited variable moisture content with relatively high values near or above the Plastic Limit. The potential also exists for wet season saturation of the Stratum I silts with attendant loss of strength and stability.

- Although specific site grading criteria is not yet available, it is understood that grading will involve placement of up to 4 ft of Select Fill. Current plans are to obtain fill soils from the 8 ft deep ± Tailwater Retention Pond. Based on conditions encountered in the two (2) borings in this area, the soils should generally be suitable for fill with selective placement of the moderate plasticity clay intervals of Strata II and III in deeper fill areas outside the main building.
- It is understood that the main building will be a pre-engineered metal structure with slab on grade and relatively light foundation loads. Considering the planned placement of several feet of fill in the building area, a foundation system consisting of individual and continuous spread footing should be appropriate. At this writing, allowable soil bearing pressures on the order of 2000 to 3000 psf should be appropriate pending final grading plans and fill soil characteristics yet to be developed.
- The potential exists for wet season loss of strength and stability of the moisture sensitive silty soils, Stratum I. Accordingly, site grading should be scheduled for typically drier late Summer/early Fall months to reduce the potential for undercut and replacement of saturated Stratum I soils. Undercut to depths on the order of 2 ft should be anticipated for wet season site grading. Specific review of undercut potential will be performed when site grading plans are available.

As noted, this is an interim report of geotechnical factors of potential consequence to final design, site grading and future performance of the Cold Steppe facility. Final recommendations will be prepared as design development continues.







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SYMBOLS AND TERMS USED ON BORING LOGS

SOIL TYPES	SAMPLER TYPES
(SHOWN IN SYMBOL COLUMN)	(SHOWN ON SAMPLES COLUMN)
Gravel Sand Silt Clay Predominant type shown heavy	Shelby Rock Split No Tube Core Spoon Recovery

TERMS DESCRIBING CONSISTENCY OR CONDITION

COARSE GRAINED SOILS(major portion retained on No. 200 sieve): Includes (1) clean gravels and sands, and (2) silty or clayey gravels and sands. Condition is rated according to relative density, as determined by laboratory tests.

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VERY LOOSE LOOSE MEDIUM DENSE DENSE VERY DENSE	5 ;	0-4 4-10 10-30 30-50 50 And above	0 - 15 % 15-35 % 35-65 % 65-85 % 85-100 %

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DESC	RIPTIVE	TERM

UNCONFINED COMPRESSIVE STRENGTH TON/SQ. FT.

VERY SOFT SOFT FIRM	Less than 0.25 0.25 - 0.50
	0.50 - 1.00 1.00 - 2.00
STIFF	2.00-4.00
VERY STIFF	4.00 and higher
HARD	

NOTE: Slickensided and fissured clays may have lower unconfined compressive strengths than shown above, because of planes of weakness or cracks in the soil. The consistency ratings of such soils are based on penetrometer readings.

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